

环向离散加筋圆柱曲板的侧压弹性稳定性

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引言

为便于分析加筋壳的总体稳定性,经常可以把它简化为按正交各向异性的连续弹性体处理。但这种近似方法只能是在一定的条件下才能成立。通过分析周边滑动筒壳,具有环向离散加筋圆柱曲板在侧压下的稳定性,在假设失稳前曲板为薄膜受力状态的前提下;本文比较了在不同参数范围内,按正交各向异性曲板计算与更精确地按环向离散加筋曲板计算的结果。从而对按正交各向异性曲板计算的适用条件提出一些看法。

目前虽有一些分析加筋圆柱壳在侧压或全压作用下的稳定性的文章,例如参考文献〔1〕、〔2〕,但有关曲板的还很少。参考文献〔3〕、〔4〕分析了单层或正交各向异性曲板的稳定性。曲板的侧压弹性稳定实验的数据及乎空白。因此,为推进航空等有关事业的发展,加强曲板的实验与理论研究是极为必要的。

符 号

x, y, z 座标,
 h, b, l 壳壁几何尺寸,
 c_1, c_2 纵、环筋的截面重心到壳壁中面距离,
 R 壳壁曲率半径,
 d_1, d_2 纵、环筋的间距,
 A_x, A_y 纵、环筋的截面面积,
 I_x, I_y 纵、环筋截面对壳壁中面的惯性矩,
 u, v, w x, y, z 方向的位移,
 $\epsilon_x, \epsilon_y, \epsilon_z$ 应变,
 $\sigma_x, \sigma_y, \tau_{xy}, \tau_{yx}$ 应力,
 n_x, n_y, n_{xy}, n_{yx} 壳壁内平面法向力、剪力,
 m_x, m_y, m_{xy}, m_{yx} 壳壁内弯矩、扭矩,
 N_x, N_y 纵、环筋的法向力(广义),
 M_x, M_y 纵、环筋的弯矩(广义),
 $\tilde{n}_x, \tilde{n}_y, \tilde{n}_{xy}, \tilde{N}_x, \tilde{N}_y$ 失稳前壳壁,纵筋、环筋内给定的平面力,

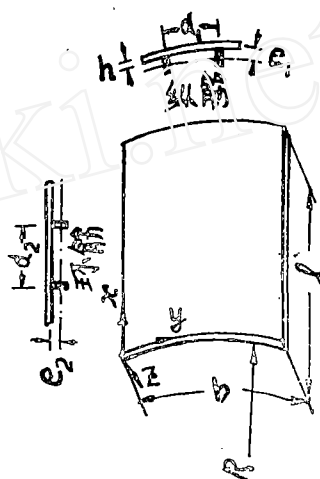


图 1

$\bar{n}_x, \bar{n}_y, \bar{n}_{xy}, \bar{n}_{yx}, \bar{N}_x, \bar{N}_y$ 边界上已知的壳壁、纵筋、环筋上的平面力,
 $\bar{m}_x, \bar{m}_y, \bar{M}_x, \bar{M}_y$ 边界上已知的壳壁、纵筋、环筋上的弯矩,
 $\bar{v}_x, \bar{v}_y, \bar{V}_x, \bar{V}_y$ 边界上已知的壳壁、纵筋、环筋上的等值剪力,
 \bar{R} 角点反力, P 横向载荷,
 Δ 广义函数(即Dirac函数)的和($\Delta = \sum_i \delta(x - x_i)$ 其中 δ 为广义函数, i 为环筋的序, x_i 为各环筋的座标位置),
 δ 变分符号,
 $E, \nu, E_x, \nu_x, E_y, \nu_y$ 壳壁,纵筋、环筋的弹性模量及波桑系数,
 D 壳壁弯曲刚度 ($D = \frac{Eh^3}{12(1-\nu^2)}$),
 B 壳壁平面刚度 ($B = \frac{Eh}{(1-\nu^2)}$),

N 环筋的数量

$$\mu_1 = \frac{E_x A_x (1 - \nu^2)}{E h d_1}, \quad \mu_2 = \frac{E_y A_y (1 - \nu^2)}{E h d_2},$$

$$\chi_1 = \frac{E_x A_x (1 - \nu^2) e_1}{E h d_1 \pi} \sqrt{\frac{B}{D}},$$

$$\chi_2 = \frac{E_y A_y (1 - \nu^2) e_2}{E h d_2 \pi} \sqrt{\frac{B}{D}},$$

$$\eta_1 = \frac{E_x I_x}{D d_1}, \quad \eta_2 = \frac{E_y I_y}{D d_2},$$

j, m, n 序数 (其中 m, n 又可分别代表纵向、环向波数),

$$\beta = \frac{b}{l} \quad (\text{宽长比}),$$

$$K_r = \frac{b^2}{\pi^2 R} \sqrt{\frac{B}{D}} \quad (\text{出率参数}),$$

$$K_p = \frac{p R b^2}{\pi^2 D} \quad (\text{临界载荷参数}),$$

$$K_A = \frac{A_y}{h d_2}.$$

(一) 基本方程

(1) 基本假设:

- ①壳、筋截面在变形前、后保持为平面,
- ②壳、筋截面内应力各为直线分布,
- ③平面剪力扭矩仅由壳壁承受,
- ④在纵筋内 $\sigma_y = 0$, 在环筋内 $\sigma_x = 0$. 在壳、筋内的横向法应力 σ_z 均为 0.

(2) 应力与广义力的关系

在壳内:

$$\sigma_x = \frac{n_x}{h} + \frac{m_x}{h^3/12} Z,$$

$$\sigma_y = \frac{n_y}{h} + \frac{m_y}{h^3/12} Z,$$

$$\tau_{xy} = \frac{n_{xy}}{h} - \frac{m_{xy}}{h^3/12} Z,$$

$$\tau_{yx} = \frac{n_{yx}}{h} + \frac{m_{yx}}{h^3/12} Z,$$

其中 $n_{xy} = n_{yx}$, $m_{xy} = -m_{yx}$.
在纵筋内:

$$\sigma_x = \left(\frac{1}{I_x - A_x e_1^2} \right) \left[\left(\frac{I_x N_x + M_x e_1}{A_x} \right) + (M_x - N_x e_1) Z \right], \quad (2)$$

在环筋内:

$$\sigma_y = \left(\frac{1}{I_y - A_y e_2^2} \right) \left[\left(\frac{I_y N_y - M_y e_2}{A_y} \right) + (M_y - N_y e_2) Z \right]. \quad (3)$$

(3) 应变与位移的关系

在壳内:

$$\left. \begin{aligned} \epsilon_x &= \frac{\partial u}{\partial x} - \frac{\partial^2 w}{\partial x^2} Z, \\ \epsilon_y &= \left(\frac{\partial v}{\partial y} - \frac{w}{R} \right) - \frac{\partial^2 w}{\partial y^2} Z, \\ \gamma_{xy} &= \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} - 2 \frac{\partial^2 w}{\partial x \partial y} Z, \end{aligned} \right\} \quad (4)$$

在纵筋内:

$$\epsilon_x = \frac{\partial u}{\partial x} - \frac{\partial^2 w}{\partial x^2} Z, \quad (5)$$

在环筋内:

$$\epsilon_y = \left(\frac{\partial v}{\partial y} - \frac{w}{R} \right) - \frac{\partial^2 w}{\partial y^2} Z. \quad (6)$$

(4) Reissner 广义变分原理 [5]

在小挠度变形的情况下, 应用 Reissner 的广义变分原理 [5] 可以推导出基本方程。变分原理的表达式是:

$$\delta \left[\int_V \int \int F dV - \int_{S_0} \int \int (\bar{p}_x u + \bar{p}_y v + \bar{p}_z w) dS_0 \right] = 0, \quad (7)$$

其中 $F = \sigma_x \epsilon_x + \sigma_y \epsilon_y + \sigma_z \epsilon_z + \tau_{xy} \gamma_{xy} + \tau_{yx} \gamma_{yx} + \tau_{yz} \gamma_{yz} - W$,
W 为以应力表示的应变能,
 S_0 为已知应力的边界,
V 为体积。

将纵筋按照应变能相等的原则折算到壳壁内, 经过详细运算, 从 (1) — (6) 式代入 (7) 式后可得到环向离散加筋圆柱曲板的以下基本方程。

平衡方程:

$$\left. \begin{aligned} \frac{\partial}{\partial x} \left(n_x + \frac{N_x}{d_1} \right) + \frac{\partial}{\partial y} n_{xy} &= 0, \\ \frac{\partial}{\partial x} n_{xy} + \frac{\partial}{\partial y} (n_y + N_y \Delta) &= 0, \\ \frac{\partial^2}{\partial x^2} \left(m_x + \frac{M_x}{d_1} \right) + \frac{\partial^2}{\partial y^2} (m_y + M_y \Delta) - \frac{\partial^2}{\partial x \partial y} m_{xy} + \frac{\partial^2}{\partial x \partial y} m_{yx} + \frac{1}{R} (n_y + N_y \Delta) + \\ + \left(\tilde{n}_x + \frac{\tilde{N}_x}{d_1} \right) \frac{\partial^2 w}{\partial x^2} + \left(\tilde{n}_y + \tilde{N}_y \Delta \right) \frac{\partial^2 w}{\partial y^2} + 2 \tilde{n}_{xy} \frac{\partial^2 w}{\partial x \partial y} + P &= 0. \end{aligned} \right\} (8)$$

广义力与广义位移的关系:

$$\left. \begin{aligned} n_x &= B \left[\frac{\partial u}{\partial x} + \nu \left(\frac{\partial v}{\partial y} - \frac{w}{R} \right) \right], \\ \frac{N_x}{d_1} &= \frac{E_x A_x}{d_1} \left[\frac{\partial u}{\partial x} - \frac{\partial^2 w}{\partial x^2} \epsilon_1 \right], \\ n_y &= B \left[\left(\frac{\partial v}{\partial y} - \frac{w}{R} \right) + \nu \frac{\partial u}{\partial x} \right], \\ N_y &= E_y A_y \left[\left(\frac{\partial v}{\partial y} - \frac{w}{R} \right) - \frac{\partial^2 w}{\partial y^2} \epsilon_2 \right], \\ n_{xy} &= \frac{B(1-\nu)}{2} \left(\frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right), \\ m_x &= -D \left(-\frac{\partial^2 w}{\partial x^2} + \nu \frac{\partial^2 w}{\partial y^2} \right), \\ \frac{M_x}{d_1} &= \frac{E_x I_x}{d_1} \left(-\frac{\partial^2 w}{\partial x^2} + \frac{A_x \epsilon_1}{I_x} \frac{\partial u}{\partial x} \right), \\ m_y &= -D \left(\frac{\partial^2 w}{\partial y^2} + \nu \frac{\partial^2 w}{\partial x^2} \right), \\ M_y &= E_y I_y \left[-\frac{\partial^2 w}{\partial y^2} + \frac{A_y \epsilon_2}{I_y} \left(\frac{\partial v}{\partial y} - \frac{w}{R} \right) \right], \\ m_{xy} = -m_{yx} &= D(1-\nu) \frac{\partial^2 w}{\partial x \partial y}. \end{aligned} \right\} (9)$$

边界条件:

$$\left. \begin{aligned} \text{在曲边 } n_x + \frac{N_x}{d_1} &= \bar{n}_x + \frac{\bar{M}_x}{d_1} \text{ 或 } u = 0, \\ n_{xy} &= \bar{n}_{xy} \text{ 或 } v = 0, \\ m_y + \frac{M_y}{d_1} &= \bar{m}_x + \frac{\bar{M}_x}{d_1} \text{ 或 } \frac{\partial w}{\partial x} = 0, \\ \frac{\partial}{\partial x} \left(m_x + \frac{M_x}{d_1} \right) + \frac{\partial m_{yx}}{\partial y} - \frac{\partial m_{xy}}{\partial y} + \left(\tilde{n}_x + \frac{\tilde{M}_x}{d_1} \right) \frac{\partial w}{\partial x} + \tilde{n}_{xy} \frac{\partial w}{\partial y} &= \left(\bar{v}_x + \frac{\bar{V}_x}{d_1} \right) \text{ 或 } w = 0, \\ \text{在直边 } n_y + N_y \Delta &= \bar{n}_y + \bar{N}_y \Delta \text{ 或 } v = 0, \end{aligned} \right\} (10)$$

$$n_{xy} = \bar{n}_{xy} \quad \text{或} \quad u = 0,$$

$$m_y + M_y \Delta = \bar{M}_y \Delta \quad \text{或} \quad \frac{\partial w}{\partial y} = 0,$$

$$\frac{\partial}{\partial y} (m_y + M_y \Delta) - \frac{\partial m_{xy}}{\partial x} + \frac{\partial m_{yx}}{\partial x} + (\tilde{n}_y + \tilde{M}_y \Delta) \frac{\partial w}{\partial y} + \tilde{n}_{xy} \frac{\partial w}{\partial x} = (\bar{v}_y + \bar{V}_y \Delta)$$

或 $w = 0$;

在角点 $m_{xy} - m_{yx} = \bar{R}$ 或 $w = 0$ 。

以上(9)式与文献[1]中的有关结果相类同。

(二) 微分方程及其解法

假定失稳前曲板为薄膜受力状态, 在受侧压时, 曲板内的给定平面力可有以下两种写法:

(1) 当横向载荷 P 为均分布时:

$$\tilde{n}_x + \frac{\tilde{N}_x}{d_1} = \tilde{n}_{xy} = 0,$$

$$\tilde{n}_y + \tilde{N}_y \Delta = -PR,$$

(11)

这也就表明, 壳壁内的平面法向力 \tilde{n}_y 是不连续的; 在附有环筋处, 壳壁内 \tilde{n}_y 的绝对值突然变小。为消除此突变, 保持薄膜受力状态, 则需在有环筋处附加环向线分布载荷, 以使环筋内的予加应力与壳壁内的相同。

(2) 当 $P = \bar{P} + \frac{\bar{P} A_y}{h} \Delta$ 时:

$$\tilde{n}_x + \frac{\tilde{N}_x}{d_1} = \tilde{n}_{xy} = 0,$$

$$\tilde{n}_y + \tilde{N}_y \Delta = -\bar{P} R \left(1 + \frac{A_y}{h} \Delta \right).$$

(12)

以下将简称(11)、(12)式所代表的情况为情况(1)、情况(2)。显见情况(1)是情况(2)的特殊情况。当线分布载荷趋于零时, 情况(2)即趋于情况(1)。为此, 以下将以情况(2)为出发点推导微分方程。

将(9)式代入(8)式中的前三个, 按照通常的作法[6], 在假定失稳前曲板为薄膜受力状态时, 可以得到计算环向离散加筋圆柱曲板的侧压稳定性的微分方程。

$$\left[(1 + \mu_1) \frac{\partial^2}{\partial x^2} + \frac{1 - \nu}{2} \frac{\partial^2}{\partial y^2} \right] u + \left[\frac{1 + \nu}{2} \frac{\partial^2}{\partial x \partial y} \right] v - \left[\frac{\nu}{R} \frac{\partial}{\partial x} + \frac{\chi_1 \pi}{\sqrt{B/D}} \frac{\partial^3}{\partial x^3} \right] w = 0, \quad (13)$$

$$\left[\frac{1 - \nu}{2} \frac{\partial^2}{\partial x \partial y} \right] u + \left[\frac{1 - \nu}{2} \frac{\partial^2}{\partial x^2} + \left(1 + \frac{E_y A_y (1 - \nu^2) \Delta}{Eh} \right) \frac{\partial^2}{\partial y^2} \right] v - \left[\frac{1}{R} \left(1 + \frac{E_y A_y (1 - \nu^2) \Delta}{Eh} \right) \right]$$

$$\frac{\partial}{\partial y} + \frac{E_y A_y (1 - \nu^2) e_2 \Delta}{Eh} \frac{\partial^3}{\partial y^3} \Big] w = 0, \quad (14)$$

$$\left[\frac{\nu}{R} \frac{\partial}{\partial x} + \frac{\chi_1 \pi}{\sqrt{B/D}} \frac{\partial^3}{\partial x^3} \right] u + \left[\frac{1}{R} \left(1 + \frac{E_y A_y (1 - \nu^2) \Delta}{Eh} \right) \frac{\partial}{\partial y} + \frac{E_y A_y (1 - \nu^2) e_2 \Delta}{Eh} \right] \frac{\partial^3}{\partial y^3} \Big] v -$$

$$-\frac{D}{B} \left[(1 + \eta_1) \frac{\partial^4}{\partial x^4} + 2 \frac{\partial^4}{\partial x^2 \partial y^2} + \left(1 + \frac{E_y I_y \Delta}{D} \right) \frac{\partial^4}{\partial y^4} + \left(\frac{\bar{P} R}{D} \left(1 + \frac{A_y}{h} \Delta \right) + \right.$$

$$+ \frac{2E_r A_r e_2}{RD} \Delta \left) \frac{\partial^2}{\partial y^2} + \frac{B}{R^2 D} \left(1 + \frac{E_r A_r (1 - \nu^2)}{Eh} \Delta \right) \right] w = 0. \quad (15)$$

令

$$\left. \begin{aligned} u &= \sum_{mn} a_{mn} u_{mn}, \\ v &= \sum_{m,n} b_{mn} v_{mn}, \\ w &= \sum_{m,n} c_{mn} w_{mn}, \end{aligned} \right\} (16)$$

其中 u_{mn} 、 v_{mn} 、 w_{mn} 为满足所有边界条件的位移函数。于是由伽辽金法可以对应 (13)、(14)、(15) 各式得到

$$\iint \left\{ \dots \right\} u_{mn} dx dy = 0, \quad (13)'$$

$$\iint \left\{ \dots \right\} v_{mn} dx dy = 0, \quad (14)'$$

$$\iint \left\{ \dots \right\} w_{mn} dx dy = 0. \quad (15)'$$

(三) 滑动简支条件下的稳定方程及其化简

$$\left. \begin{aligned} \text{此时, 在曲边上: } & N_x = 0, v = 0, \\ & M_x = 0, w = 0, \\ \text{在直边上: } & N_y = 0, u = 0, \\ & M_y = 0, w = 0, \end{aligned} \right\} (17)$$

令

$$\left. \begin{aligned} u &= \sum_{m,n} a_{mn} \cos \frac{m\pi x}{l} \sin \frac{n\pi y}{b}, \\ v &= \sum_{m,n} b_{mn} \sin \frac{m\pi x}{l} \cos \frac{n\pi y}{b}, \\ w &= \sum_{m,n} c_{mn} \sin \frac{m\pi x}{l} \sin \frac{n\pi y}{b}. \end{aligned} \right\} (18)$$

为便于计算, 取环向加筋为等间距的情况, 则将 (18) 代入 (13)'、(14)'、(15)', 即得计算稳定性的代数方程组:

$$\left[1 + \frac{1 - \nu}{2} \frac{m^2 \beta^2}{n^2} - \frac{\left(\frac{1 + \nu}{2} \right)^2 m^2 \beta^2}{(1 + \mu_1) m^2 \beta^2 + \frac{1 - \nu}{2} n^2} \right] b_{mn}' + \mu_2 \sum_j \delta_{mj} b_{jn}' + \frac{1}{n} \left[K_r + \right.$$

$$+ \frac{\left(\frac{1+\nu}{2} m^2 \beta^2\right) \left(\pi \chi_1 m^2 \beta^2 - \nu K_r\right)}{(1+\mu_1) m^2 \beta^2 + \left(\frac{1-\nu}{2}\right) n^2} \left. c_{mn} + \frac{1}{n} (\mu_2 K_r - \pi \chi_2 n^2) \sum_j \delta_{mj} c_{jn} = 0, \quad (19) \right\}$$

$$\frac{1}{n} \left\{ K_r + \frac{\left(\frac{1+\nu}{2} m^2 \beta^2\right) \left(\pi \chi_1 m^2 \beta^2 - \nu K_r\right)}{(1+\mu_1) m^2 \beta^2 + \left(\frac{1-\nu}{2}\right) n^2} \right\} b_{mn}' + \frac{1}{n} (\mu_2 K_r - \pi \chi_2 n^2) \sum_j \delta_{mj} b_{jn}' + \frac{1}{n^2} \left\{ (1+\eta_1) m^4 \beta^4 + 2m^2 n^2 \beta^2 + n^4 + K_r^2 - \frac{m^2 \beta^2 (\pi \chi_1 m^2 \beta^2 - \nu K_r)^2}{(1+\mu_1) m^2 \beta^2 + \frac{1-\nu}{2} n^2} \right\} c_{mn} + \left\{ \eta_2 n^2 - 2\pi \chi_2 K_r + \frac{1}{n^2} \mu_2 K_r^2 \right\} \sum_j \delta_{mj} c_{jn} - \left(\frac{1}{1 + \frac{N}{N+1} K_A} \right) K_P c_{mn} - \left(\frac{K_A}{1 + \frac{N}{N-1} K_A} \right) K_P \sum_j \delta_{mj} c_{jn} = 0, \quad (20)$$

其中 $b_{mn}' = \frac{b}{\pi} \sqrt{\frac{B}{D}} b_{mn}$,

$$\delta_{mj} = \begin{cases} 1 & \text{当 } (j-m) = 2k(N+1) \text{ 时 } k=0, 1, 2, \dots \\ -1 & \text{当 } (j+m) = 2k(N+1) \text{ 时 } k=1, 2, 3, \dots \\ 0 & \text{其他} \end{cases}$$

在(20)式中已考虑了使前节中情况(1),情况(2)的总载荷量相等,以便于计算比较(此时

$$\bar{P} = \left(\frac{1}{1 + \frac{N}{N+1} K_A} \right) P), \text{ 如令 } K_A = 0 \text{ 即为情况(1), 否则为情况(2).}$$

以上(10)、(20)式可缩写为矩阵向量型式:

$$\left. \begin{aligned} [V_1] \bar{v} + [W_1] \bar{w} &= 0, \\ [V_2] \bar{v} + [W_2] \bar{w} &= K_P [P] \bar{w}, \end{aligned} \right\} \quad (21)$$

其中

$$\bar{V} = \begin{pmatrix} b_{1n} \\ b_{2n} \\ b_{3n} \\ \vdots \end{pmatrix}, \quad \bar{W} = \begin{pmatrix} c_{1n} \\ c_{2n} \\ c_{3n} \\ \dots \end{pmatrix},$$

又 $[V_1]$ 、 $[V_2]$ 、 $[W_1]$ 、 $[W_2]$ 、 $[P]$ 为(19)、(20)式中有关的系数所组成的矩阵。于是可归并为:

$$[P]^{-1} \{ [W_2] - [V_2][V_1]^{-1}[W_1] \} \bar{w} = K_P \bar{w}. \quad (22)$$

以下的问题就是求解(22)式的最小特征值 K_P 及相应的特征向量 \bar{w} 。

在参考文献[6]中给出了使类似于(22)式中的矩阵降阶的办法,最后可简化为计算 $(N+1)$ 个子矩阵问题,从中可以即刻得到

(1) 加筋曲板按正交各向异性理论计算的总体失稳的公式为:

$$K_P = \frac{1}{n^2} \left\{ \left[(1+\eta_1) m^4 \beta^4 + 2m^2 n^2 \beta^2 + (1+\eta_2) n^4 \right] - \frac{\pi^2}{\frac{1-\nu}{2} (1+\mu_1) m^4 \beta^4 + \left[(1+\mu_1)(1+\mu_2) - \nu \right] m^2 n^2 \beta^2 + \frac{1-\nu}{2} (1+\mu_2) n^4} \left[\frac{1-\nu}{2} \chi_1^2 m^8 \beta^8 + \dots \right] \right\}$$

$$\begin{aligned}
 & + (1 + \mu_2) \chi_1^2 m^6 n^2 \beta^6 - \nu(1 - \nu) \frac{\chi_1 K_r}{\pi} m^6 \beta^6 - (1 + \nu) \chi_1 \chi_2 m^4 n^4 \beta^4 + (1 - \nu) \frac{K_r}{\pi} \left((1 + \mu_1) \chi_2 + \right. \\
 & \left. + (1 + \mu_2) \chi_1 \right) m^4 n^2 \beta^4 - \frac{1 - \nu}{2} \left((1 + \mu_1)(1 + \mu_2) - \nu^2 \right) \frac{K_r^2}{\pi^2} m^4 \beta^4 - \nu(1 - \nu) \frac{\chi_2 K_r}{\pi} m^2 n^4 \beta^2 + \\
 & \left. + (1 + \mu_1) \chi_2^2 m^2 n^6 \beta^2 + \frac{1 - \nu}{2} \chi_2^2 n^6 \right\} \quad (23)
 \end{aligned}$$

(2) 在环筋之间的局部失稳的公式为:

$$\begin{aligned}
 K_P = \frac{1}{n^2} \left\{ \left[(1 + \eta_1) m^4 \beta^4 + 2 m^2 n^2 \beta^2 + n^4 \right] - \right. \\
 \left. - \frac{\pi^2}{\left(\frac{1 - \nu}{2} \right) (1 + \mu_1) m^4 \beta^4 + (1 + \mu_1 - \nu) m^2 n^2 \beta^2 + \frac{1 - \nu}{2} n^4} \left[\frac{1 - \nu}{2} \chi_1^2 m^6 \beta^6 + \right. \right. \\
 \left. \left. + \chi_1^2 m^6 n^2 \beta^6 - \nu(1 - \nu) \frac{\chi_1 K_r}{\pi} m^6 \beta^6 + (1 - \nu) \frac{\chi_1 K_r}{\pi} m^4 n^2 \beta^4 - \frac{1 - \nu}{2} (1 + \mu_1 - \nu^2) \right. \right. \\
 \left. \left. \frac{K_r^2}{\pi^2} m^4 \beta^4 \right] \right\} \quad (24)
 \end{aligned}$$

(四) 计算结果与分析

下面以周边滑动筒支、受均布侧压为例,分析具有环向离散加筋圆柱曲板的弹性稳定性。经过计算环筋之间壳壁的局部失稳载荷,并将按环向离散方法与按正交各向异性方法总体失稳载荷作了比较之后,在所给参数范围内,可以得出以下看法。

(1) 按照情况(1)与情况(2)两种情况的计算比较说明:在总载荷量相等的条件下,在壳壁局部失稳之前,二者差别不大。在按照《表1》所给的局部失稳载荷以后,《表2》中的 K_P 值为《表1》中

的 $\left(1 + \frac{N}{N+1} K_A\right)$ 倍。原因是,按情况(2)计算时,当环筋间的均布载荷达到使壳壁局部失稳时,连同环筋上的线分布载荷,其总载荷量恰为在情况(1)下局部失稳时的总载荷量的 $\left(1 + \frac{N}{N+1} K_A\right)$ 倍。从而

使离散方法计算的总体失稳载荷也提高 $\left(1 + \frac{N}{N+1} K_A\right)$

倍。为简便起见,以下均按情况(1)作比较分析。

(2) 考虑环筋法向力的作用对计算总体失稳的影响极微。比较《表1》、《表3》可见,其差别仅为1%左右。

(3) 由《表4》、《表5》可见,一般地说,环筋在曲板的内侧(χ_2 为正)较在外侧(χ_2 为负)为佳。

(4) 比较《表4》、《表6》、《表7》、《表8》可说,曲率参数的影响较大。 K_r 愈大,承载能力愈高。

(5) 综合以上各表,可归纳出以下一点,即局部失稳的载荷可作为按正交各向异性方法与按离散方法计算总体失稳的分界限。在此之前二者是一致的。

(6) 加纵筋后,对提高总体失稳载荷的作用虽不大但由于它提高了局部失稳的载荷,从而扩大了按正交各向异性方法处理问题的适用范围。(见《表8》、《表10》)。

(7) 随着宽长比 β 的增大, K_P 值也不断提高。(见表(11))

以上各条看法中,第5条是最主要的。也就回答了本文一开始所提出的问题。

«表1» $K_r = 1.5 \times 10^2$ $\beta = 1$

$\eta_2 \backslash N$	1	2	3	4	5	∞
1	36.20(1)(4)	37.46(1)(4)	37.83(1)(4)	38.02(1)(4)	38.14(1)(4)	38.36(1)(4)
2	46.48(1)(4)	50.66(1)(3)	50.86(1)(3)	50.98(1)(3)	51.06(1)(3)	51.26(1)(3)
5	55.02($\frac{1}{8}$)(6)	74.37(1)(3)	76.05(1)(3)	76.75(1)(3)	77.22(1)(3)	78.22(1)(3)
10	55.53($\frac{1}{8}$)(6)	82.68($\frac{2}{4}$)(7)	114.5($\frac{3}{5}$)(7)	117.4(1)(3)	119.2(1)(3)	123.22(1)(3)
20	55.79($\frac{1}{8}$)(6)	83.01($\frac{2}{4}$)(7)	115.3($\frac{3}{5}$)(7)	152.3($\frac{4}{6}$)(8)	196.1($\frac{5}{7}$)(9)	213.2(1)(3)
50	55.95($\frac{1}{8}$)(6)	83.20($\frac{2}{4}$)(7)	115.8($\frac{3}{5}$)(7)	152.7($\frac{4}{6}$)(8)	196.5($\frac{5}{7}$)(9)	405.2(1)(2)
100	56.00($\frac{1}{8}$)(6)	83.27($\frac{2}{4}$)(7)	116.0($\frac{3}{5}$)(7)	152.8($\frac{4}{6}$)(8)	196.6($\frac{5}{7}$)(9)	605.2(1)(2)
200	56.03($\frac{1}{8}$)(6)	83.20($\frac{2}{4}$)(7)	116.0($\frac{3}{5}$)(7)	152.9($\frac{4}{6}$)(8)	196.7($\frac{5}{7}$)(9)	1005(1)(2)
400	56.04($\frac{1}{8}$)(6)	83.32($\frac{2}{4}$)(7)	116.1($\frac{3}{5}$)(7)	152.9($\frac{4}{6}$)(8)	196.7($\frac{5}{7}$)(9)	1805(1)(2)
1000	56.05($\frac{1}{8}$)(6)	83.33($\frac{2}{4}$)(7)	116.1($\frac{3}{5}$)(7)	152.9($\frac{4}{6}$)(8)	196.7($\frac{5}{7}$)(9)	4205(1)(2)
	48.78(2)(5)	78.36(3)(6)	110.8(4)(7)	148.2(5)(8)	192.2(6)(9)	

«表2» $K_r = 1.5 \times 10^2$ $\beta = 1$ $K_A = 0.45$

$\eta_2 \backslash N$	1	2	3	4	5
1	32.27(1)(4)	34.33(1)(4)	35.35(1)(4)	35.95(1)(4)	36.35(1)(4)
2	43.32(1)(3)	46.02(1)(3)	47.34(1)(3)	48.18(1)(3)	48.71(1)(3)
5	64.63(1)(3)	69.45(1)(3)	71.70(1)(3)	73.05(1)(3)	73.96(1)(3)
10	67.78($\frac{1}{8}$)(6)	106.3(1)(3)	110.9(1)(3)	113.7(1)(3)	115.5(1)(3)
20	68.33($\frac{1}{8}$)(6)	107.8($\frac{2}{4}$)(3)	154.2($\frac{3}{5}$)(7)	190.9(1)(3)	195.5(1)(3)
50	68.53($\frac{1}{8}$)(6)	108.1($\frac{2}{4}$)(7)	154.8($\frac{3}{5}$)(7)	207.6($\frac{4}{6}$)(8)	270.1($\frac{5}{7}$)(9)
100	68.60($\frac{1}{8}$)(6)	108.2($\frac{2}{4}$)(7)	155.1($\frac{3}{5}$)(7)	207.9($\frac{4}{6}$)(8)	270.4($\frac{5}{7}$)(9)
200	68.63($\frac{1}{8}$)(6)	108.2($\frac{2}{4}$)(7)	155.1($\frac{3}{5}$)(7)	207.9($\frac{4}{6}$)(8)	270.4($\frac{5}{7}$)(9)
400	68.64($\frac{1}{8}$)(6)	108.3($\frac{2}{4}$)(7)	155.2($\frac{3}{5}$)(7)	207.9($\frac{4}{6}$)(8)	270.4($\frac{5}{7}$)(9)
1000	68.66($\frac{1}{8}$)(6)	108.3($\frac{2}{4}$)(7)	155.2($\frac{3}{5}$)(7)	207.9($\frac{4}{6}$)(8)	270.4($\frac{5}{7}$)(9)

《表3》 $K_r = 1.5 \times 10^2$ $\beta = 1$ $\mu_2 = 0.5$

$\eta_2 \backslash N$	1	2	3	4	5	∞
1	36.53(1)(4)	37.83(1)(4)	38.15(1)(4)	38.28(1)(4)	38.35(1)(4)	38.47(1)(4)
2	47.21(1)(4)	51.30(1)(3)	51.43(1)(3)	51.48(1)(3)	51.51(1)(3)	51.56(1)(3)
5	55.08($\frac{1}{3}$)(6)	76.69(1)(3)	77.51(1)(3)	77.89(1)(3)	78.11(1)(3)	78.56(1)(3)
10	55.59($\frac{1}{3}$)(6)	82.82($\frac{2}{4}$)(7)	115.0($\frac{3}{5}$)(7)	120.1(1)(3)	121.2(1)(3)	123.5(1)(3)
20	55.85($\frac{1}{3}$)(6)	83.15($\frac{2}{4}$)(7)	115.8($\frac{3}{5}$)(7)	152.6($\frac{4}{6}$)(8)	196.3($\frac{5}{7}$)(9)	213.5(1)(3)
50	56.01($\frac{1}{3}$)(6)	83.84($\frac{2}{4}$)(7)	116.2($\frac{3}{5}$)(7)	153.0($\frac{4}{6}$)(8)	196.7($\frac{5}{7}$)(9)	405.6(1)(2)
100	56.06($\frac{1}{3}$)(6)	83.41($\frac{2}{4}$)(7)	116.3($\frac{3}{5}$)(7)	153.1($\frac{4}{6}$)(8)	196.3($\frac{5}{7}$)(9)	605.6(1)(2)
200	56.09($\frac{1}{3}$)(6)	83.41($\frac{2}{4}$)(7)	116.5($\frac{3}{5}$)(7)	153.2($\frac{4}{6}$)(8)	196.9($\frac{5}{7}$)(9)	1005(1)(2)
400	56.10($\frac{1}{3}$)(6)	83.46($\frac{2}{4}$)(7)	116.5($\frac{3}{5}$)(7)	153.2($\frac{4}{6}$)(8)	196.9($\frac{5}{7}$)(9)	1805(1)(2)
1000	56.11($\frac{1}{3}$)(6)	83.47($\frac{2}{4}$)(7)	116.6($\frac{3}{5}$)(7)	153.2($\frac{4}{6}$)(8)	197.0($\frac{5}{7}$)(9)	4205(1)(2)
局部失稳	48.78(4)(5)	78.36(3)(6)	110.8(4)(7)	148.2(5)(8)	192.2(6)(9)	

《表4》 $K_r = 1.5 \times 10^2$ $\beta = 1$ $\mu_2 = 0.5$ $\chi_2 = +0.35$

$\eta_2 \backslash N$	1	2	3	4	5	∞
2	28.69(1)(2)	30.58(1)(4)	52.54(1)(4)	34.41(1)(4)	36.14(1)(4)	44.44(1)(4)
5	54.81($\frac{1}{3}$)(5)	68.59(1)(3)	69.84(1)(3)	70.78(1)(3)	71.61(1)(3)	75.33(1)(3)
10	55.59($\frac{1}{3}$)(6)	82.83($\frac{2}{4}$)(7)	115.2($\frac{3}{5}$)(7)	117.1(1)(3)	117.9(1)(3)	120.3(1)(3)
20	55.87($\frac{1}{3}$)(6)	83.20($\frac{2}{4}$)(7)	115.9($\frac{3}{5}$)(7)	152.7($\frac{4}{6}$)(8)	196.4($\frac{5}{7}$)(9)	210.3(1)(3)
50	56.02($\frac{1}{3}$)(6)	83.37($\frac{2}{4}$)(7)	116.3($\frac{3}{5}$)(7)	153.1($\frac{4}{6}$)(8)	196.8($\frac{5}{7}$)(9)	405.1(1)(2)
100	56.07($\frac{1}{3}$)(6)	83.42($\frac{2}{4}$)(7)	116.5($\frac{3}{5}$)(7)	153.2($\frac{4}{6}$)(8)	196.3($\frac{5}{7}$)(9)	605.1(1)(2)
200	56.09($\frac{1}{3}$)(6)	83.45($\frac{2}{4}$)(7)	116.5($\frac{3}{5}$)(7)	153.2($\frac{4}{6}$)(8)	196.9($\frac{5}{7}$)(9)	1005.1(1)(2)
400	56.10($\frac{1}{3}$)(6)	83.46($\frac{2}{4}$)(7)	116.5($\frac{3}{5}$)(7)	153.3($\frac{4}{6}$)(8)	197.0($\frac{5}{7}$)(9)	1805(1)(2)
1000	56.11($\frac{1}{3}$)(6)	83.47($\frac{2}{4}$)(7)	116.6($\frac{3}{5}$)(7)	153.3($\frac{4}{6}$)(8)	197.0($\frac{5}{7}$)(9)	4205(1)(2)
局部失稳	48.78(2)(5)	78.35(3)(6)	110.8(4)(7)	148.2(5)(8)	192.2(6)(9)	

《表5》 $K_r = 1.5 \times 10^2$ $\beta = 1$ $\mu_2 = 0.5$ $\chi_2 = -0.35$

$\eta_2 \backslash N$	1	2	3	4	5	∞
2	24.59(1)(4)	25.55(1)(4)	26.83(1)(4)	28.21(1)(4)	29.65(1)(4)	37.80(1)(4)
5	52.18(1)(5)	57.86(1)(3)	59.09(1)(3)	60.25(1)(3)	61.40(1)(3)	66.51(1)(3)
10	55.37($\frac{1}{3}$)(6)	82.48($\frac{2}{4}$)(7)	97.49(1)(3)	100.0(1)(3)	102.3(1)(3)	111.5(1)(3)
20	55.77($\frac{1}{3}$)(6)	83.02($\frac{2}{4}$)(7)	115.4($\frac{3}{5}$)(7)	152.3($\frac{4}{6}$)(8)	180.2(1)(3)	201.5(1)(3)
50	55.98($\frac{1}{3}$)(6)	83.30($\frac{2}{4}$)(7)	116.1($\frac{3}{5}$)(7)	152.9($\frac{4}{6}$)(8)	196.6($\frac{5}{7}$)(9)	399.0(1)(3)
100	56.05($\frac{1}{3}$)(6)	83.39($\frac{2}{4}$)(7)	116.4($\frac{3}{5}$)(7)	153.1($\frac{4}{6}$)(8)	196.8($\frac{5}{7}$)(9)	399.0(1)(2)
200	56.08($\frac{1}{3}$)(6)	83.43($\frac{2}{4}$)(7)	116.5($\frac{3}{5}$)(7)	153.2($\frac{4}{6}$)(8)	196.9($\frac{5}{7}$)(9)	999.0(1)(2)
400	56.10($\frac{1}{3}$)(6)	83.46($\frac{2}{4}$)(7)	116.5($\frac{3}{5}$)(7)	153.2($\frac{4}{6}$)(8)	196.9($\frac{5}{7}$)(9)	1799(1)(2)
1000	56.11($\frac{1}{3}$)(3)	83.47($\frac{2}{4}$)(7)	116.6($\frac{3}{5}$)(7)	153.3($\frac{4}{6}$)(8)	197.0($\frac{5}{7}$)(9)	4199(1)(2)
局部失稳	48.78(2)(5)	78.36(3)(6)	110.8(4)(7)	148.2(5)(8)	192.2(6)(9)	

《表6》 $K_r = 0.75 \times 10^2$ $\beta = 1$ $\mu_2 = 0.5$ $\chi_2 = +0.35$

$\eta_2 \backslash N$	1	2	3	4	5	∞
2	21.20(1)(3)	22.78(1)(3)	24.20(1)(3)	25.43(1)(3)	26.11(1)(3)	29.28(1)(3)
5	40.33(1)(4)	50.48(1)(3)	52.50(1)(3)	53.53(1)(3)	54.19(1)(3)	56.28(1)(3)
10	41.39($\frac{1}{3}$)(5)	64.29($\frac{2}{4}$)(5)	91.81($\frac{3}{5}$)(6)	93.37(1)(2)	93.57(1)(2)	94.12(1)(2)
20	41.65($\frac{1}{3}$)(5)	65.13($\frac{2}{4}$)(5)	92.52($\frac{3}{5}$)(6)	127.5($\frac{4}{6}$)(7)	133.4(1)(2)	134.1(1)(2)
50	41.78($\frac{1}{3}$)(5)	65.57($\frac{2}{4}$)(5)	92.89($\frac{3}{5}$)(6)	127.9($\frac{4}{6}$)(7)	171.0($\frac{5}{7}$)(8)	254.1(1)(2)
100	41.82($\frac{1}{3}$)(5)	65.66($\frac{2}{4}$)(6)	93.01($\frac{3}{5}$)(6)	128.0($\frac{4}{6}$)(7)	171.1($\frac{5}{7}$)(8)	454.1(1)(2)
200	41.84($\frac{1}{5}$)(5)	65.69($\frac{2}{4}$)(6)	93.06($\frac{3}{5}$)(6)	128.1($\frac{4}{6}$)(7)	171.1($\frac{5}{7}$)(8)	851.1(1)(2)
400	41.85($\frac{1}{3}$)(5)	65.70($\frac{2}{4}$)(6)	93.09($\frac{3}{5}$)(6)	128.1($\frac{4}{6}$)(7)	171.2($\frac{5}{7}$)(8)	1654(1)(2)
1000	41.86($\frac{1}{3}$)(5)	65.71($\frac{2}{4}$)(6)	93.11($\frac{3}{5}$)(6)	128.1($\frac{4}{6}$)(7)	171.2($\frac{5}{7}$)(8)	2279(1)(1)
局部失稳	37.42(2)(5)	60.18(3)(5)	88.19(4)(6)	123.3(5)(7)	165.6(6)(7)	

《表7》 $K_1 = 2.5 \times 10^2$ $\beta = 1$ $\mu_2 = 0.5$ $\chi_2 = +0.35$

$\eta_2 \backslash N$	1	2	3	4	5	∞
2	36.81(1)(4)	37.96(1)(4)	39.52(1)(4)	41.22(1)(4)	42.88(1)(4)	54.50(1)(4)
5	68.98($\frac{1}{3}$)(6)	88.98(1)(4)	91.68(1)(4)	93.34(1)(4)	94.62(1)(4)	102.5(1)(4)
10	70.74($\frac{1}{3}$)(7)	102.1($\frac{2}{4}$)(8)	139.3($\frac{3}{5}$)(9)	157.9(1)(3)	158.6(1)(3)	163.1(1)(3)
20	71.03($\frac{1}{3}$)(7)	102.5($\frac{2}{4}$)(8)	139.7($\frac{3}{5}$)(9)	181.8($\frac{4}{6}$)(9)	228.2($\frac{5}{7}$)(10)	253.1(1)(3)
50	71.18($\frac{1}{7}$)(7)	102.6($\frac{2}{4}$)(8)	139.9($\frac{3}{5}$)(9)	182.2($\frac{4}{6}$)(9)	228.6($\frac{5}{7}$)(10)	523.1(1)(3)
100 ^a	71.23($\frac{1}{3}$)(7)	102.7($\frac{2}{4}$)(8)	139.9($\frac{3}{5}$)(9)	182.4($\frac{4}{6}$)(9)	228.7($\frac{5}{7}$)(10)	961.6(1)(2)
200	71.25($\frac{1}{3}$)(7)	102.7($\frac{2}{4}$)(8)	140.0($\frac{3}{5}$)(9)	182.4($\frac{4}{6}$)(9)	228.7($\frac{5}{7}$)(10)	1361(1)(2)
400	71.26($\frac{1}{3}$)(7)	102.7($\frac{2}{4}$)(8)	140.0($\frac{3}{5}$)(9)	182.4($\frac{4}{6}$)(9)	228.7($\frac{5}{7}$)(10)	2161(1)(2)
1000	71.27($\frac{1}{3}$)(7)	102.8($\frac{2}{4}$)(8)	140.0($\frac{3}{5}$)(9)	182.5($\frac{4}{6}$)(9)	228.8($\frac{5}{7}$)(10)	4561(1)(2)
局部失稳	59.79(2)(6)	95.81(3)(7)	134.5(4)(8)	176.6(5)(9)	223.6(6)(10)	

《表8》 $K_1 = 5.0 \times 10^2$ $\beta = 1$ $\mu_2 = 0.5$ $\chi_2 = +0.35$

$\eta_2 \backslash N$	1	2	3	4	5	∞
2	49.70(1)(5)	50.56(1)(5)	51.70(1)(5)	53.28(1)(5)	55.11(1)(5)	77.92(1)(5)
5	95.37($\frac{1}{3}$)(8)	126.6(1)(4)	127.8(1)(4)	129.0(1)(4)	130.2(1)(4)	144.8(1)(4)
10	96.96($\frac{1}{3}$)(8)	138.2($\frac{2}{4}$)(9)	185.5($\frac{3}{5}$)(10)	213.2(1)(4)	214.4(1)(4)	224.8(1)(4)
20	97.56($\frac{1}{3}$)(8)	139.0($\frac{2}{4}$)(9)	185.7($\frac{3}{5}$)(10)	236.9($\frac{4}{6}$)(11)	292.3($\frac{5}{7}$)(12)	384.8(1)(4)
50	97.87($\frac{1}{3}$)(8)	139.5($\frac{2}{4}$)(9)	186.1($\frac{3}{5}$)(10)	237.3($\frac{4}{6}$)(11)	292.6($\frac{5}{7}$)(12)	717.6(1)(3)
100	97.97($\frac{1}{3}$)(8)	139.4($\frac{2}{4}$)(9)	186.3($\frac{3}{5}$)(10)	237.3($\frac{4}{6}$)(11)	292.7($\frac{5}{7}$)(12)	1167(1)(3)
200	98.01($\frac{1}{3}$)(8)	139.6($\frac{2}{4}$)(9)	186.3($\frac{3}{5}$)(10)	237.4($\frac{4}{6}$)(11)	292.8($\frac{5}{7}$)(12)	2067(1)(3)
400	98.04($\frac{1}{3}$)(8)	139.6($\frac{2}{4}$)(9)	186.4($\frac{3}{5}$)(10)	237.5($\frac{4}{6}$)(11)	292.8($\frac{5}{7}$)(12)	3328(1)(2)
1000	98.05($\frac{1}{3}$)(3)	139.6($\frac{2}{4}$)(9)	186.4($\frac{3}{5}$)(10)	237.5($\frac{4}{6}$)(11)	292.8($\frac{5}{7}$)(12)	6228(1)(2)
局部失稳	83.02(2)(7)	127.2(3)(9)	176.6(4)(10)	229.7(5)(11)	286.4(6)(14)	

《表9》 $K_r = 1.5 \times 10^2$ $\beta = 1$ $\mu_2 = 0.5$ $\chi_2 = 0.35$ $\eta_1 = 1.00 \times 10^2$

$r_2 \backslash N$	1	2	3	4	5	∞
2	37.58(1)(4)	40.26(1)(4)	42.65(1)(4)	44.58(1)(4)	46.04(1)(4)	51.50(1)(4)
5	81.60(1)(4)	85.06(1)(3)	86.58(1)(3)	87.84(1)(3)	88.37(1)(3)	90.61(1)(3)
10	124.9(1)(3)	129.8(1)(3)	131.6(1)(3)	132.6(1)(3)	133.4(1)(3)	135.6(1)(3)
20	135.5($\frac{1}{3}$)(8)	217.6(1)(3)	221.0(1)(3)	222.5(1)(3)	223.3(1)(3)	225.6(1)(3)
50	137.0($\frac{1}{3}$)(8)	252.0($\frac{2}{4}$)(11)	408.8($\frac{3}{5}$)(14)	457.8(1)(2)	458.0(1)(2)	458.5(1)(2)
100	137.4($\frac{1}{3}$)(8)	252.6($\frac{2}{4}$)(11)	409.5($\frac{3}{5}$)(14)	609.5($\frac{4}{6}$)(17)	657.7(1)(2)	658.5(1)(2)
200	137.6($\frac{1}{3}$)(8)	252.8($\frac{2}{4}$)(11)	409.8($\frac{3}{5}$)(14)	609.9($\frac{4}{6}$)(17)	853.5($\frac{5}{7}$)(20)	1058(1)(2)
400	137.7($\frac{1}{3}$)(8)	253.0($\frac{2}{4}$)(11)	410.0($\frac{3}{5}$)(14)	610.0($\frac{4}{6}$)(17)	853.7($\frac{5}{7}$)(20)	1858(1)(2)
1000	137.8($\frac{1}{3}$)(8)	253.0($\frac{2}{4}$)(11)	410.1($\frac{3}{5}$)(14)	610.1($\frac{4}{6}$)(17)	853.8($\frac{5}{7}$)(20)	4258(1)(2)
局部失稳	12.82(2)(7)	201.5(3)(11)	355.0(4)(13)	553.3(5)(16)	796.1(6)(19)	

《表10》 $K_r = 1.5 \times 10^2$ $\beta = 1$ $\mu_2 = 0.5$ $\chi_2 = +0.35$ $\eta_2 = 2.00 \times 10^2$

$r_2 \backslash N$	1	2	3	4	5	∞
2	43.88(1)(4)	46.56(1)(4)	48.95(1)(4)	50.86(1)(4)	52.32(1)(4)	57.75(1)(4)
5	87.57(1)(4)	94.43(1)(4)	96.99(1)(4)	98.77(1)(3)	99.49(1)(3)	101.7(1)(3)
10	137.2(1)(3)	141.0(1)(3)	142.7(1)(3)	143.7(1)(3)	144.5(1)(3)	146.7(1)(3)
20	185.0($\frac{1}{3}$)(9)	229.8(1)(3)	232.4(1)(3)	233.7(1)(3)	234.4(1)(3)	236.7(1)(3)
50	187.5($\frac{1}{3}$)(9)	346.0($\frac{2}{4}$)(13)	482.5(1)(2)	482.8(1)(2)	483.0(1)(2)	483.5(1)(2)
100	188.5($\frac{1}{3}$)(9)	346.8($\frac{2}{4}$)(13)	562.7($\frac{3}{5}$)(16)	682.5(1)(2)	682.9(1)(2)	683.5(1)(2)
200	188.6($\frac{1}{3}$)(10)	347.1($\frac{2}{4}$)(13)	563.3($\frac{3}{5}$)(16)	838.1($\frac{4}{6}$)(20)	1082(1)(2)	1083(1)(2)
400	188.8($\frac{1}{3}$)(10)	347.3($\frac{2}{4}$)(13)	563.5($\frac{3}{5}$)(16)	838.4($\frac{4}{6}$)(20)	1174($\frac{5}{7}$)(23)	1883(1)(2)
1000	188.8($\frac{1}{3}$)(10)	347.4($\frac{2}{4}$)(13)	563.7($\frac{3}{5}$)(16)	838.5($\frac{4}{6}$)(20)	1174($\frac{5}{7}$)(23)	4283(1)(2)
局部失稳	123.6(2)(8)	274.5(3)(1)	486.1(4)(15)	759.2(5)(19)	1093(6)(23)	

《表11》 $K_r = 1.5 \times 10^2$ $\mu_2 = 0.5$

	η_2	$\beta = 0.5$		$\beta = 1.5$		$\beta = 2.0$	
		$\chi_2 = -0.35$	$\chi_2 = +0.35$	$\chi_2 = -0.35$	$\chi_2 = +0.35$	$\chi_2 = -0.35$	$\chi_2 = +0.35$
总体失稳 $N = \infty$	1	11.00(1)(3)	14.58(1)(3)	35.08(1)(5)	43.21(1)(5)	47.87(1)(5)	56.97(1)(5)
	2	20.00(1)(3)	23.58(1)(3)	53.64(1)(4)	62.82(1)(4)	72.87(1)(5)	81.97(1)(5)
	5	35.50(1)(2)	42.14(1)(3)	101.6(1)(4)	110.8(1)(4)	137.8(1)(4)	143.9(1)(4)
	10	55.50(1)(2)	62.14(1)(2)	181.6(1)(4)	187.8(1)(3)	217.8(1)(4)	223.9(1)(4)
	20	95.50(1)(2)	102.1(1)(2)	271.7(1)(3)	277.8(1)(3)	377.8(1)(4)	383.9(1)(4)
	50	215.5(1)(2)	222.1(1)(2)	541.7(1)(3)	547.8(1)(3)	675.3(1)(3)	665.9(1)(3)
	100	415.5(1)(2)	422.1(1)(2)	991.7(1)(3)	997.8(1)(3)	1125(1)(3)	1115(1)(3)
	200	815.5(1)(2)	822.1(1)(2)	1466(1)(2)	1444(1)(2)	2025(1)(3)	2015(1)(3)
	400	1195(1)(1)	1201(1)(1)	2244(1)(2)	2244(1)(2)	2943(1)(2)	2871(1)(2)
	1000	1795(1)(1)	1801(1)(1)	4666(1)(2)	4644(1)(2)	5343(1)(2)	5271(1)(2)
局部失稳	$N = 1$	22.38(2)(4)		78.36(2)(6)		110.8(2)(7)	
	$N = 2$	35.12(3)(5)		128.8(3)(8)		192.2(3)(9)	
	$N = 3$	48.78(4)(5)		192.2(4)(9)		299.2(4)(10)	
	$N = 4$	61.68(5)(6)		269.9(5)(10)		436.6(5)(12)	
	$N = 5$	78.36(6)(6)		363.6(6)(11)		604.6(6)(13)	

表中给出 K_r 值。括弧中的数值是失稳时的波型(m)、(n)。(m)在前,(n)在后。(m)中所给出的是失稳时的主要纵向波数。

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